


RESEARCH ARTICLE

A hypoplastic model for gas hydrate-bearing sandy sediments

Xuhui Zhang^{1,2}  | Jia Lin³ | Xiaobing Lu^{1,2} | Lele Liu⁴ | Changling Liu⁴ | Mingyao Li⁵ | Yewang Su^{5,2,6}

¹Key Laboratory for Mechanics in Fluid Solid Coupling Systems, Institute of Mechanics, Chinese Academy of Sciences, Beijing 100190, China

²School of Engineering Science, University of Chinese Academy of Sciences, Beijing 100049, China

³University of Natural Resources and Life Sciences, Vienna, Austria

⁴Qingdao Institute of Marine Geology, Qingdao, China

⁵State Key Laboratory of Nonlinear Mechanics, Institute of Mechanics, Chinese Academy of Sciences, Beijing 100190, China

⁶State Key laboratory of Hydrosience and Engineering, Tsinghua University, Beijing 100084, China

Correspondence

X. H. Zhang, Key Laboratory for Mechanics in Fluid Solid Coupling Systems, Institute of Mechanics, Chinese Academy of Sciences, Beijing 100190, China.

Email: zhangxuhui@imech.ac.cn

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Summary

Gas hydrate-bearing sediments (GHBSs) have been considered as a potential energy resource. In this paper, the mechanical properties of GHBS are firstly investigated by the integrated test apparatus for synthesis of GHBS using silty sand as skeleton. Triaxial tests indicate an obvious transition of stress-strain relationship from strain hardening under low hydrate saturation and strain softening under high hydrate saturation. The hypoplastic models coupled with Drucker-Prager criterion and the Mohr-Coulomb criterion are proposed to analyze the stress-strain relationship of GHBS with considering the effective porosity because of the hydrate filling in the pores of GHBS. The strain hardening and softening behaviors are well predicted with less material parameters compared with the classical models. Compared with the test results, the proposed hypoplastic models are verified to be capable of capturing the salient features of the mechanical behaviors of GHBS under the conditions of little temperature change and no hydrate dissociation.

KEYWORDS

effective porosity, gas hydrate-bearing sediment, hypo-plastic model, strain hardening, strain softening

1 | INTRODUCTION

As the most common naturally occurring gas hydrate, gas hydrate-bearing sediments (GHBSs) have been considered as a potential energy resource, and the related exploration and exploitation technologies have been extensively investigated in many countries. Gas hydrate-bearing sediment, a crystalline solid composed of natural gas and hydrogen-bonded water

molecules, is typically formed under the relatively low temperature and high pressure conditions and naturally exists in the permafrost regions and continental offshore worldwide.¹ However, the stability and behaviors of GHBS are strongly dependent on thermohydromechanical conditions or human activities during the extraction, and the dissociation of hydrates may cause massive seafloor instabilities and engineering geological problems, such as submarine landslide, borehole instability, and foundation failures. The proper understanding of the behavior of GHBS related to hydrate formation and dissociation is significant to the drilling and production applications of the large amount of the potential energy resource.²⁻⁴

The laboratory studies have been comprehensively conducted using reconstituted samples and focus on triaxial compression tests to investigate the mechanical properties of GHBS. The behavior of GHBS is complex in natural and the effects of hydrate saturation, temperature, and effective confining pressure on the modulus and the shear strength have been studied mainly by using reconstituted hydrate-bearing sands.^{1,5-7} The experimental data show that with the increase of hydrate saturation and effective confining pressure, the elastic modulus and shear strength are generally increased. Strain softening occurs at high hydrate saturation (typically about 25% to 45%) while plastic and strain hardening is observed at low hydrate saturation.^{1,8} The temperature below freezing point leads to the formation of frozen GHBS (composed of ice, hydrate, and silt sand), and the mechanical behavior strengthens with the increase of the total saturation of hydrate and ice.⁹ The tetrahydrofuran hydrate-bearing silt clay shows typical ductile behavior, and the shear strength increases linearly with the increase of hydrate saturation and confining pressure.¹⁰

Several models have been proposed to formulate the stress-strain relationship of GHBS.^{11,12} The elastic modulus of gas hydrate-bearing coarse sand is formulated by series and parallel model,¹³ and the shear strength of tetrahydrofuran hydrate-bearing clay is analyzed on the basis of the Mohr-Coulomb and Drucker-Prager criterion.¹⁰ The Duncan-Chang model is applied to simulate the nonlinear elastic behavior of GHBS.¹⁴⁻¹⁶ The distinct element model is adopted to describe the cohesion and friction between grains and hydrate in GHBS.¹⁷ The modified Cam-Clay model is used to take into account the shearing and volumetric dilation of hydrate-bearing sands. The temperature and hydrate saturation can be introduced into these models; however, the number of the material parameters increase, and the strain softening and dilatant behavior of GHBS are still difficult to take into account at the same time.

In most of the abovementioned constitutive models, it is assumed that the deformation of GHBS is infinitesimal, and the strain can be decomposed into elastic and plastic parts and modeled according to the theory of elastoplasticity. For the purpose of simple and practical in the engineering applications, a hypoplastic model is proposed to describe the 3-dimensional nonlinear stress-strain behaviors of GHBS. Taking advantage of the hypoplastic model, it is not necessary to consider the yield surface, plastic potential, flow and hardening rules, and decomposition of deformation. The hypoplastic model is based on the nonlinear tensor functions and is capable of capturing the salient features of granular materials, in particular for the problems involving extremely low stress or extremely high stress levels,¹⁸⁻²¹ while the hypoplastic model does not take into account the coupling between mechanics, fluid, and heat.

Because the mechanical behavior of unsaturated GHBS is investigated under the conditions of little temperature change and no hydrate dissociation, the coupling between mechanics, fluid, and heat is not considered. The objective of this paper is to present a hypoplastic model of GHBS, understand the mechanism of strain softening at high hydrate saturation, and validate the model by experiments capturing the main composition and geotechnical condition of natural GHBS. Firstly, hypoplastic models are recapitulated within the theoretical framework of hypoplasticity and proposed to formulate the mechanical behaviors of GHBS on the basis of experimental investigation. The shear strength is modeled on the basis of the modified Drucker-Prager criterion and Mohr-Coulomb criterion, respectively. The effects of critical void ratio and effective porosity under different hydrate saturations are taking into account in the proposed hypoplastic model to describe the strain softening behavior. Secondly, the experimental apparatus and procedures are fully presented, and a set of triaxial compression tests are conducted on reconstituted GHBS by using silt sand as skeleton under various hydrate saturations and effective confining pressures. Finally, the proposed model is applied to describe the strain hardening and softening behaviors and compared with the experimental results under various hydrate saturations and confining pressures.

2 | A HYPOPLASTIC MODEL FOR GHBS

As an alternative approach to the general elastoplastic models, a simple hypoplastic constitutive model is proposed to describe the mechanical behavior of GHBS as follows²⁰:

$$\dot{\sigma} = c_1(\text{tr}\sigma)\dot{\epsilon} + c_2 \frac{\text{tr}(\sigma\dot{\epsilon})\sigma}{\text{tr}\sigma} + \left(c_3 \frac{\sigma^2}{\text{tr}\sigma} + c_4 \frac{(\sigma^*)^2}{\text{tr}\sigma} \right) \|\dot{\epsilon}\|, \quad (1)$$

where $\dot{\sigma}$ and $\dot{\epsilon}$ are the rates of stress and strain tensors; $\|\dot{\epsilon}\| = \sqrt{\text{tr}\dot{\epsilon}^2}$ denotes the Euclidean norm; and c_1, c_2, c_3, c_4 are dimensionless material parameters, which are independent of the invariants and joint invariants of σ and $\dot{\epsilon}$. The deviatoric stress σ^* is defined as

$$\sigma^* = \sigma - \frac{1}{3}(\text{tr}\sigma)\mathbf{I}, \quad (2)$$

where σ is the stress tensor and \mathbf{I} is the unit tensor.

To obtain a feasible constitutive equation, the parameters are required to be calibrated according to largely various laboratory tests. To this end, we chose the most widely used triaxial tests, and the matrices of $\sigma, \dot{\epsilon}$, and $\dot{\sigma}$ can be expressed as follows:

$$\sigma = \begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}, \dot{\epsilon} = \begin{bmatrix} \dot{\epsilon}_1 & 0 & 0 \\ 0 & \dot{\epsilon}_2 & 0 \\ 0 & 0 & \dot{\epsilon}_3 \end{bmatrix}, \dot{\sigma} = \begin{bmatrix} \dot{\sigma}_1 & 0 & 0 \\ 0 & \dot{\sigma}_2 & 0 \\ 0 & 0 & \dot{\sigma}_3 \end{bmatrix}. \quad (3)$$

The 4 coefficients of constitutive equation proposed in Equation 1 can then be determined by substituting the corresponding matrices according to the triaxial compression tests. The differential equations are obtained as follows:

$$\dot{\sigma}_1 = c_1(\sigma_1 + 2\sigma_3)\dot{\epsilon}_1 + c_2 \frac{\sigma_1\dot{\epsilon}_1 + 2\sigma_3\dot{\epsilon}_3}{\sigma_1 + 2\sigma_3} \sigma_1 + \left[c_3\sigma_1^2 + \frac{4}{9}c_4(\sigma_1 - \sigma_3)^2 \right] \frac{\sqrt{\dot{\epsilon}_1^2 + 2\dot{\epsilon}_3^2}}{\sigma_1 + 2\sigma_3}, \quad (4)$$

$$\dot{\sigma}_3 = c_1(\sigma_1 + 2\sigma_3)\dot{\epsilon}_3 + c_2 \frac{\sigma_1\dot{\epsilon}_1 + 2\sigma_3\dot{\epsilon}_3}{\sigma_1 + 2\sigma_3} \sigma_3 + \left[c_3\sigma_3^2 + \frac{4}{9}c_4(\sigma_1 - \sigma_3)^2 \right] \frac{\sqrt{\dot{\epsilon}_1^2 + 2\dot{\epsilon}_3^2}}{\sigma_1 + 2\sigma_3}. \quad (5)$$

The above 2 equations are 2 linear ones with 4 unknown material parameters (c_1, c_2, c_3, c_4), which can be obtained by solving the equation system. To this end, the 4 parameters can be related to some well-established parameters in soil mechanics by considering some specific stress states, for example, the hydrostatic and the stress state at failure. The detail information is referred to the work of Wu and Bauer,²⁰ and the explicit expressions of c_1, c_2, c_3, c_4 are given as

$$\begin{aligned} c_1 &= \frac{E_i}{3\sigma_3(1 + \nu_i)}, \\ c_2 &= \frac{9d_f\nu_i(R_f^2 - 4) + d_i(2 + R_f)^2(1 + 4\nu_f)}{d_f(1 - 2\nu_i)(R_f^2 - 4) + d_i(2\nu_f - R_f)(R_f - 4)} c_1, \\ c_3 &= \frac{9\nu_i(2\nu_f - R_f)(R_f - 4) - (1 - 2\nu_i)(2 + R_f)^2(1 + 4\nu_f)}{d_f(1 - 2\nu_i)(R_f^2 - 4) + d_i(2\nu_f - R_f)(R_f - 4)} c_1, \\ c_4 &= \frac{9 \left[(R_f^2\nu_f + 1)(2 + R_f)^2 c_1 + (R_f - 1)R_f(2\nu_f - R_f)c_2 \right]}{d_f(R_f - 1)^2(R_f^2 - 4)}, \end{aligned} \quad (6)$$

where the initial tangent modulus $E_i = [(\dot{\sigma}_1 - \dot{\sigma}_3)/\dot{\epsilon}_1]_{R=1}$, the initial Poisson ratio $\nu_i = [\dot{\epsilon}_3/\dot{\epsilon}_1]_{R=1}$, the failure stress ratio $R_f = [\sigma_1/\sigma_3]_{\max}$, and the failure ratio $\nu_f = (\dot{\epsilon}_3/\dot{\epsilon}_1)_{R=R_f}$, in which $R = \sigma_1/\sigma_3$ being the stress ratio with $d_i = \sqrt{1 + 2\nu_i^2}$ and $d_f = \sqrt{1 + 2\nu_f^2}$.

For the purpose of simplicity in engineering practice, the former expressions can be further simplified in view of the triaxial compression tests. Extensive experiments on various granular materials have verified that the radial strain at the initiation of triaxial compression tests can be regarded as zero, ie, $\nu_i = 0$.²⁰ Hence, the expression in Equation 6 can be simplified as

$$\begin{aligned}
c_1 &= \frac{E_i}{3\sigma_3}, \\
c_2 = c_3 &= \frac{(R_f + 2)^2(1 + 4\nu_f)}{d_f(R_f^2 - 4) + (2\nu_f - R_f)(R_f - 4)} c_1, \\
c_4 &= -\frac{9(2 + R_f)^2 [R_f^2 d_f \nu_f + R_f \nu_f (2\nu_f - R_f) + d_f + (2\nu_f - R_f)]}{d_f(R_f - 1)^2 [d_f(R_f^2 - 4) + (2\nu_f - R_f)(R_f - 4)]} c_1.
\end{aligned} \tag{7}$$

As long as c_1 is determined by fitting the initial tangent modulus of a triaxial compression test, other parameters can be then identified by the failure stress ratio R_f and the failure Poisson ratio ν_f accordingly, such as the Drucker-Prager criterion and Mohr-Coulomb criterion. It is noted that the 4 material coefficients can also be calibrated with other sort of tests and more refined identification can be obtained.

To this end, the constitutive model can be expressed explicitly for a specific kind of materials with the former procedure. Now, the stress can be then updated for a time step Δt by integrating the calibrated equations 4 and 5 to get the initial stress for the next time step as follows:

$$\boldsymbol{\sigma}(t + \Delta t) = \boldsymbol{\sigma}(t) + \int_t^{t+\Delta t} \dot{\boldsymbol{\sigma}}[\boldsymbol{\sigma}(\eta), \dot{\boldsymbol{\epsilon}}(\eta)] d\eta. \tag{8}$$

To solve the above integral equation, numerical methods are required instead of the analytical solution and a simple 1-step, Euler forward scheme is applied in this paper.

At this point, the entire hypoplastic constitutive model with only 4 material parameters is well achieved, and it is worth noting that many conventional elastoplastic concepts in soil mechanics can be related to this constitutive model. As stated above, the 4 material parameters can be identified by fitting of largely experimental data coupling with some failure criterion, for instance, the Drucker-Prager criterion and Mohr-Coulomb criterion. As is known that the shear strength of the DHBS is strongly affected by degrees of hydrate saturation, the modified Drucker-Prager criterion and the Mohr-Coulomb criterion are adopted by considering the hydrate saturation. The modified Drucker-Prager criterion and the Mohr-Coulomb criterion can introduce the hydrate saturation into the material parameters and describe the shear strength well in the first-order approximation. It is noted that the failure criterion of the Drucker-Prager and the Mohr-Coulomb is equivalent if the yield line of the Drucker-Prager criterion is internally tangent to that of the Mohr-Coulomb criterion in the deviatoric plane under the plain strain condition.¹⁰

2.1 | The shear strength modeled by the Drucker-Prager criterion (Model I)

Taking into account the effects of hydrate saturation, the Drucker-Prager criterion takes the modified form as

$$\sqrt{J_2} = K_f(S_h) + \beta(S_h) \cdot I_1, \tag{9}$$

where $J_2 = [(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2]/6$, $I_1 = \sigma_1 + \sigma_2 + \sigma_3$, the material parameters $K_f(S_h)$ and $\beta(S_h)$ are the functions of hydrate saturation S_h . For the case of triaxial compression tests, the second principle stress equals to the third principle stress, ie, $\sigma_2 = \sigma_3$, and the modified Drucker-Prager criterion can be simplified as

$$\sigma_1 - \sigma_3 = \sqrt{3}K_f(S_h) + \sqrt{3}\beta(S_h) \cdot (\sigma_1 + 2\sigma_3). \tag{10}$$

The form can be further expressed as

$$\sigma_1 - \sigma_3 = A(S_h) + B(S_h) \cdot \sigma_3, \tag{11}$$

where $A(S_h) = \frac{\sqrt{3}K_f(S_h)}{1 - \sqrt{3}\beta(S_h)}$, $B(S_h) = \frac{3\sqrt{3}\beta(S_h)}{1 - \sqrt{3}\beta(S_h)}$.

As taking the first-order approximation, Equation 11 can be simplified as

$$\sigma_1 - \sigma_3 = (A'(0) \cdot S_h + A(0)) + (B'(0) \cdot S_h + B(0)) \cdot \sigma_3, \tag{12}$$

where $A'(0) = \partial A(S_h = 0)/\partial S_h$, $B'(0) = \partial B(S_h = 0)/\partial S_h$.

Following the work of Rowe²² for the relationship between stress and dilatancy of granular materials under undrained and static loading condition, the ratio of the lateral strain to axial strain, ie, $\dot{\epsilon}_3/\dot{\epsilon}_1$, can be derived from the ratio of axial stress to lateral stress, ie, σ_1/σ_3 , and the friction angle between the soil grains. The relationship can be expressed as follows:

$$\frac{\dot{\epsilon}_3}{\dot{\epsilon}_1} = \frac{\sigma_1}{\sigma_3 \tan^2(45 + \phi_u/2)}, \quad (13)$$

where the ratio of axial stress to lateral stress at the failure state is defined as the shear failure parameter R_f and takes form as

$$R_f = \left(\frac{\sigma_1}{\sigma_3} \right)_{\max} \quad (14)$$

and the failure Poisson ratio ν_f can be then obtained by $\nu_f = [\dot{\epsilon}_3/\dot{\epsilon}_1]_{R=R_f}$ with $d_f = \sqrt{1 + 2\nu_f^2}$. Fine sands are used for the reconstituted hydrate samples in this study, and the friction angle is chosen as 26° , and the secant modulus is adopted. At this point, as long as the hydrate saturation of GHBS is given, the 4 material parameters of the proposed constitutive model (see Equation 7) are determined and the stress-strain relationship of GHBS is eventually obtained.

2.2 | The shear strength modeled by the Mohr-Coulomb criterion (Model II)

As a granular material, the cohesion between soil grains and hydrate is significant in GHBS. Hence, the Mohr-Coulomb criterion is adopted to study the influence of the effective confining pressure, internal friction, and cohesion. At this stage, the failure stress ratio R_f is related to the friction angle ϕ as follows:

$$R_f = \frac{1 + \sin\phi}{1 - \sin\phi}, \quad (15)$$

and the failure Poisson ratio ν_f can be determined by experimental data or roughly estimated with the dilatancy theory in the work of Rowe.²² Therefore, the hypoplastic model can be modified as

$$\dot{\sigma} = c_1(\text{tr}(\sigma-c))\dot{\epsilon} + c_2 \frac{\text{tr}((\sigma-c)\dot{\epsilon})(\sigma-c)}{\text{tr}(\sigma-c)} + \left(c_3 \frac{(\sigma-c)^2}{\text{tr}(\sigma-c)} + c_4 \frac{((\sigma-c)^*)^2}{\text{tr}(\sigma-c)} \right) \|\dot{\epsilon}\|, \quad (16)$$

where c is the cohesion of GHBS.

2.3 | The hypoplastic model for the strain softening of GHBS (Model III)

Because of the increase of the hydrate, the cementation among the grains of GHBS gets strengthened and the cohesion gets increased, and the strain softening occurs as the hydrate saturation reaches a critical extent.²¹ The effect of the porosity is obviously observed in various laboratory tests of soil mechanics. The preliminary physical explanation can be drawn for GHBS that as the hydrate occupies the pores of GHBS, the effective porosity decreases, and the filling of the pores and compression state leads to nontrivial rotation, slide, and rearrangement among soil grains, and the shear dilatancy occurs at the critical void ratio of GHBS. Hence, we propose a new hypoplastic constitutive model to describe the strain softening behavior as follows:

$$\dot{\sigma} = c_1(\text{tr}(\sigma-c))\dot{\epsilon} + c_2 \frac{\text{tr}((\sigma-c)\dot{\epsilon})(\sigma-c)}{\text{tr}(\sigma-c)} + \left(c_3 \frac{(\sigma-c)^2}{\text{tr}(\sigma-c)} + c_4 \frac{((\sigma-c)^*)^2}{\text{tr}(\sigma-c)} \right) \|\dot{\epsilon}\| \cdot I_e, \quad (17)$$

where the density function $I_e = (a - 1)D_c + 1$ with a material constant $I_e|_{e=e_{\min}} = a$ and the modified relative density $D_c = (e_{\text{crit}} - e)/(e_{\text{crit}} - e_{\min})$, in which e is the void ratio and e_{crit} and e_{\min} are the critical and minimum void ratio, respectively. The void ratio e can be updated during a deformation process according to the evolution equation $\dot{e} = (1 + e)trD$ with a specific initial void ratio at any well-defined stress state, for example, as a function of porosity and hydrate saturation $e = \varphi(1 - S_n)/(1 - \varphi(1 - S_n))$.

3 | STRESS-STRAIN MEASUREMENTS OF GHBS

3.1 | Experimental apparatus

Figure 1 shows the diagram of the experimental apparatus used for synthesis of GHBS and the triaxial compression tests. The apparatus is an integrated testing machine with a capacity of the effective confining pressures ranging from 0 to 14 MPa with an accuracy of 0.5%, and the temperatures can be controlled in the range of -20°C to 20°C with an accuracy of 2.5%. The apparatus can also provide a maximum backpressure by methane gas-supply cylinder from 0 to 10 MPa for achieving the formation of methane hydrate-bearing sediments under the necessary high pressure and low temperature conditions. The total gas percolating can be measured with a gas flow meter. The GHBS can be then formed under proper pressure and temperature. More detailed information of the apparatus is referred to the previous work.⁹

Gas hydrate-bearing sediments are sediment of soil (sand, silt sand, clay, etc) mixed with gas, water, and gas hydrate in nature. The hydrate occurrence types are disseminated and pore-filling in coarse sands and veined or nodule in fine-grained sediments. Exploration for gas hydrate as an energy resource focuses on sandy GHBS.¹ Here, sandy GHBS samples were prepared to capture the main composition and geotechnical condition.

3.2 | Host specimen material

The host specimen was prepared by fine silty sand, and the grain size distribution is illustrated in Figure 2. The specific gravity is about 2.69, and the relative density is 54%. The dry density of the specimen is 1.6 g/cm^3 with a porosity of around 40%.

3.3 | Experimental procedure

Gas hydrate-bearing sediments are found to distribute in permafrost and marine sediments in nature, where the temperature is above zero and the pressure is above phase equilibrium pressure of gas hydrate. The stiffness and failure strength of GHBS depend on the magnitude of temperature, and the temperature drop (from 5°C to 1°C) leads to the increase of the stiffness and failure strength because of temperature dependence of the strength of the hydrate/sediment contact.²³ Hence, in all the tests, the temperature was kept at 1°C during GHBS preparation, and the triaxial compressions were conducted with little temperature change and no hydrate dissociation.

The unsaturated GHBS was prepared under the condition of partial saturated water in the pores. The gas saturated method was applied to obtain uniform gas hydrate distribution.^{23,24} The procedures were (1) certain water (about 20 mL) was injected into the specimen. (2) The gas was supplied under the pressure of 3 MPa and the temperature was turned to 1°C , and the total gas volume was controlled according to the theoretical volume of reacted gas into hydrate

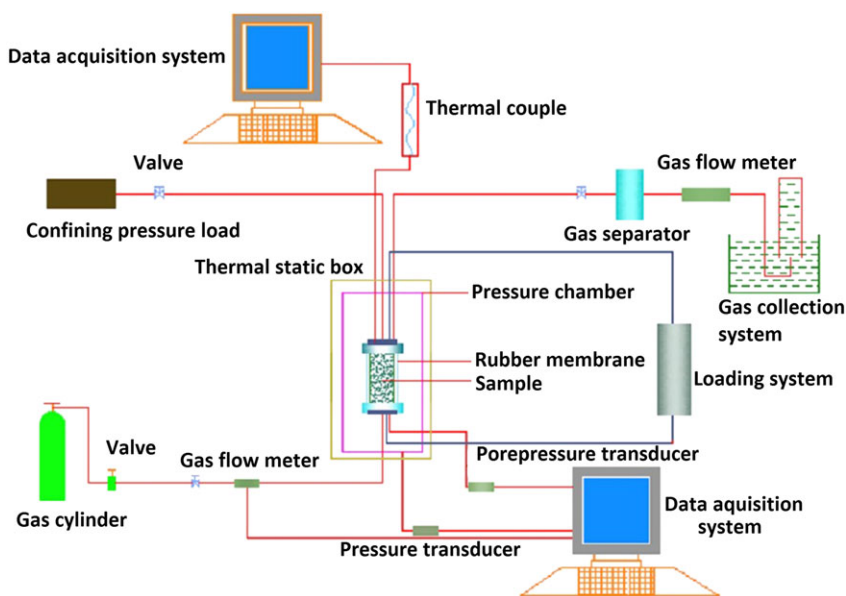


FIGURE 1 Integrated test apparatus for synthesis of gas hydrate-bearing sediment and triaxial compression tests [Colour figure can be viewed at wileyonlinelibrary.com]

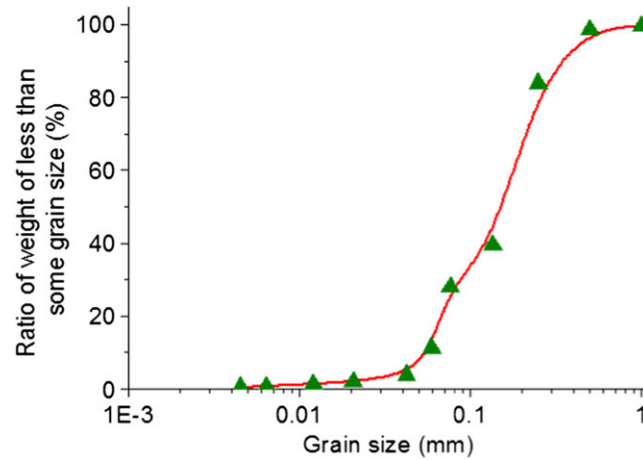


FIGURE 2 Grain size distribution of fine silty sand [Colour figure can be viewed at wileyonlinelibrary.com]

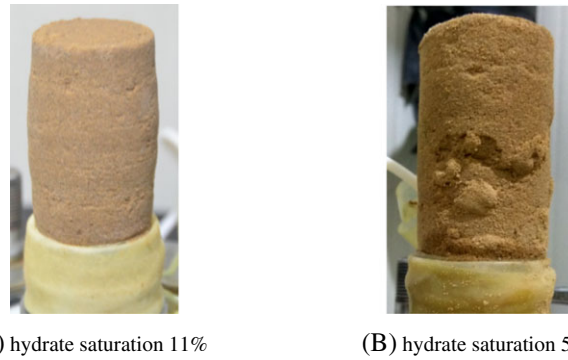
and free gas in the pores. (3) As the GHBS preparation was completed, the confining pressures were applied with 4, 6, and 8 MPa. Because of the 3 MPa of pore gas pressure, the effective confining pressures were 1, 3, and 5 MPa accordingly. (4) The temperature was remain to 0°C to 1°C, and the average hydrate saturations were 5%, 11%, 23%, 40%, and 52%. The hydrate saturation was controlled by the methane gas pressure and the total gas volume, and the amount of gas used in GHBS formation was calculated on the basis of ideal gas equation and checked by collected gas during hydrate dissociation after triaxial tests. The mechanical properties of the reconstituted GHBS were obtained under the backpressure 3 MPa and drained condition and listed in Table 1, such as modulus, shear strength, cohesion, and internal friction angle. The initial modulus was determined by $E = (\Delta\sigma/\Delta\varepsilon)|_{\varepsilon = 1\%}$, and the maximum deviatoric stress was obtained until the engineering strain reached 15% (plastic failure) or the strain at failure. The cohesion and internal friction angle of GHBS with certain hydrate saturation were achieved through 3 experiments under different effective confining pressures.

4 | RESULTS AND DISCUSSION

On the basis of a series of tests, the stress-strain curves under different saturations and effective confining pressures are shown in Figure 5 (test points of stress with strain). It is noted that the stress-strain curves are obviously nonlinear and no apparent elastic parts are observed.

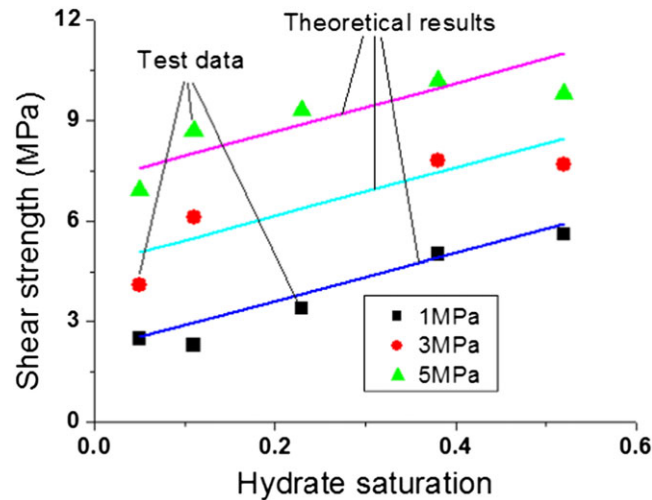
TABLE 1 Mechanical parameters of gas hydrate-bearing sediment

Hydrate Saturation, %	Effective Confining Pressure σ_3 , MPa	Initial Modulus, MPa	The Maximum Axial Stress σ_{1max} , MPa	Cohesion, MPa	Internal Friction Angle, °
5	1	90	3.5	0.3	22
	3	100	7.1		
	5	140	11.5		
11	1	112	3.2	0.4	26
	3	250	9.1		
	5	254	13.7		
23	1	170	4.4	0.8	26
	3	324	10.9		
	5	356	14.3		
40	1	132	6.0	1.1	24
	3	192	10.7		
	5	362	15.1		
52	1	316	6.6	1.5	22
	3	374	10.7		
	5	450	14.8		



(A) hydrate saturation 11%

(B) hydrate saturation 52%

FIGURE 3 The shape of samples after tests [Colour figure can be viewed at wileyonlinelibrary.com]**FIGURE 4** Shear strength under different hydrate saturations and effective confining pressures [Colour figure can be viewed at wileyonlinelibrary.com]

During the initial stage of compression, uniform deformation is predominant. Under low hydrate saturation ($\leq 40\%$), the shape of the samples changes from a cylinder into a drum-like, and the stress-strain behaves as strain hardening; under high hydrate saturation, strain softening behavior occurs, and a small shear crack at the side of the samples appears as in Figure 3. Then strain localization will occur and develop into a shear band from the crack.

To validate the proposed model for GHBS, the shear strength under different hydrate saturations and effective pressures is predicted with the proposed theoretical model using Equation 12. The parameters used in the model are

TABLE 2 Parameters used in Model I

Hydrate Saturation, %	Effective Confining Pressure, MPa	C1	C2	C3	C4
5	1	-11	-121	-121	424
	3	-9	-229	-229	1219
	5	-7	-193	-193	1076
11	1	-26	-327	-327	1226
	3	-14	-187	-187	735
	5	-8	-139	-139	606
23	1	-28	-229	-229	704
	3	-18	-182	-182	623
	5	-13	-200	-200	833
40	1	-22	-135	-135	374
	3	-11	-111	-111	383
	5	-12	-164	-164	647

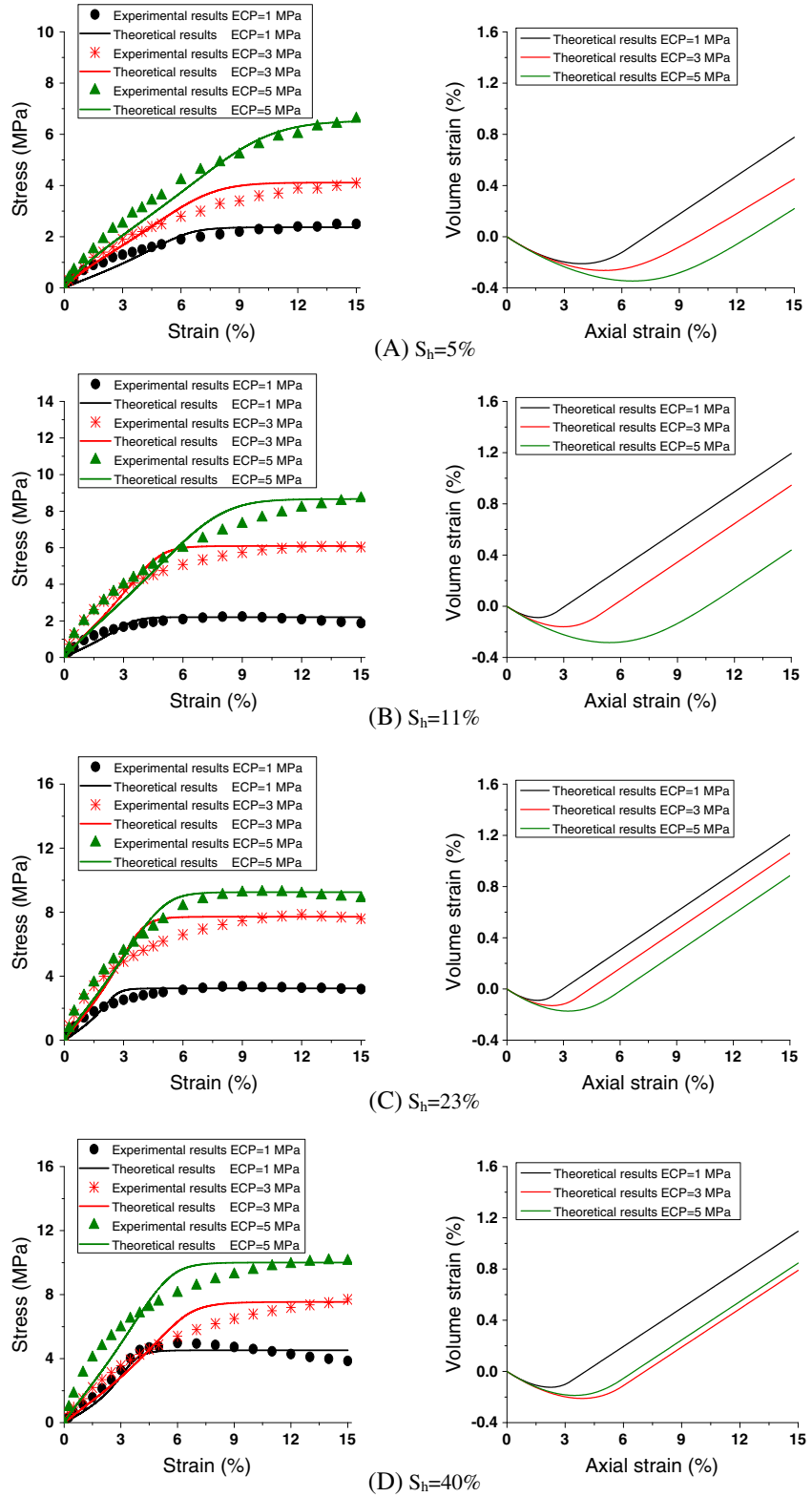


FIGURE 5 Stress-strain curves under different saturations and effective confining pressures [Colour figure can be viewed at wileyonlinelibrary.com]

as follows: $A'(0) = 7.13$, $A(0) = 0.92$, $B'(0) = 0.02$, $B(0) = 1.26$, and the relative coefficient R^2 is set to be 0.92. As shown in Figure 4, the theoretical results are in good agreement with the test data.

The material parameters used in Model I (see in Equations 11 and 12) are then calculated according to the mechanical parameters of GHBS in Table 1 and shown in Table 2. The stress-strain curves and volume strains are simulated under the different hydrate saturations, ie, 5%, 11%, 23%, and 40%, and compared with the experimental data (see

TABLE 3 Parameters used in Model II

Hydrate Saturation, %	Effective Confining Pressure, MPa	C1	C2	C3	C4
5	1	-11	-398	-398	2405
	3	-9	-325	-325	1968
	5	-7	-253	-253	1530

Figure 5). It can be seen from the figures that the results are in good agreement with the test results and the plastic behaviors are clearly observed under low hydrate saturation, which indicates the effectiveness of the proposed Model I, while the theoretical volume change indicates that shear dilation occurs because the hypoplastic model may have a shortcoming in overestimation of dilatancy.

In Table 2, the material parameters C2/C1 and C4/C1 are different under different effective confining pressures, because different confining pressures lead to different failure stress ratios, and the material parameters C2 and C4 are nonlinear with the failure stress ratio.

The material parameters in Model II are calculated and listed in Table 3 according to the cohesion, internal friction angle, and hydrate saturation S_h . Under 5% hydrate saturation, the shear strength is modeled by the Mohr-Coulomb criterion. The strain hardening behaviors under the effective confining pressures of 1, 3, and 5 MPa are predicted and compared with the experimental results, and the theoretical volume change indicates that the shear dilation is not pronounced as shown in Figure 6.

The material parameters in Model III are calculated and listed in Table 4, and strain softening behavior is simulated under high hydrate saturation 52%. The material parameters used in the experiments are as follows: Considering the effect of hydrate saturation to the effective porosity, the critical void ratio e_{crit} and the minimum void ratio e_{min} are 0.31 and 0.18, respectively, and the parameter a is set to be 0.85. As can be seen from Figure 7 that the stress-strain results obtained by Model III are in good agreement with the experimental results. The important characteristics of strain softening behavior are well predicted under high hydrate saturation (52%), the volume strain decreases with the increase of the effective confining pressure, and the theoretical volume change indicates that the shear dilation is more pronounced than of that lower hydrate saturation.

For hydrate saturation 5%, the critical void ratio e_{crit} and the minimum void ratio e_{min} are 0.42 and 0.70, respectively, and the parameter a is set to be 0.85. The material parameters C1~C4 are adopted in Table 2. As can be seen from

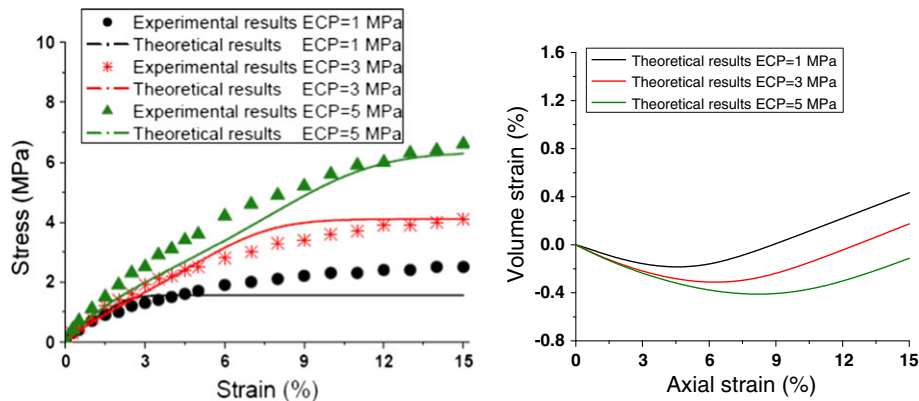


FIGURE 6 Stress-strain curves under the hydrate saturation of 5% in Model II [Colour figure can be viewed at wileyonlinelibrary.com]

TABLE 4 Parameters used in Model III

Hydrate Saturation, %	Effective Confining Pressure, MPa	C1	C2	C3	C4
50	1	-50	-287	-287	775
	3	-33	-346	-346	1197
	5	-30	-448	-448	1851

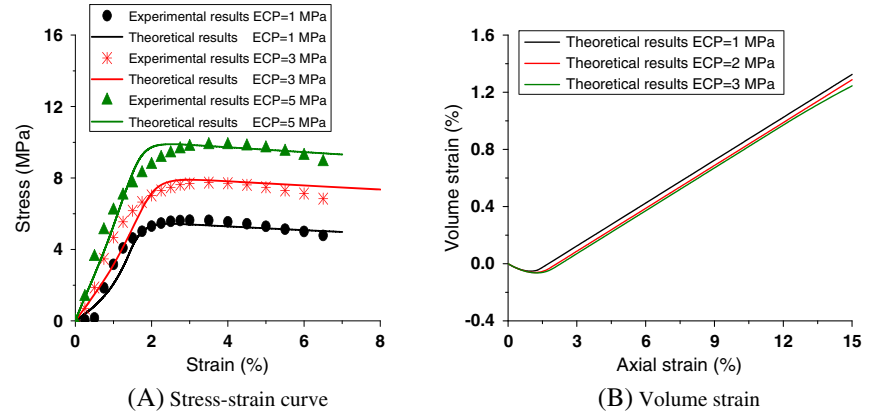


FIGURE 7 Stress-strain curves and shear dilation in Model III ($S_h = 52\%$) [Colour figure can be viewed at wileyonlinelibrary.com]

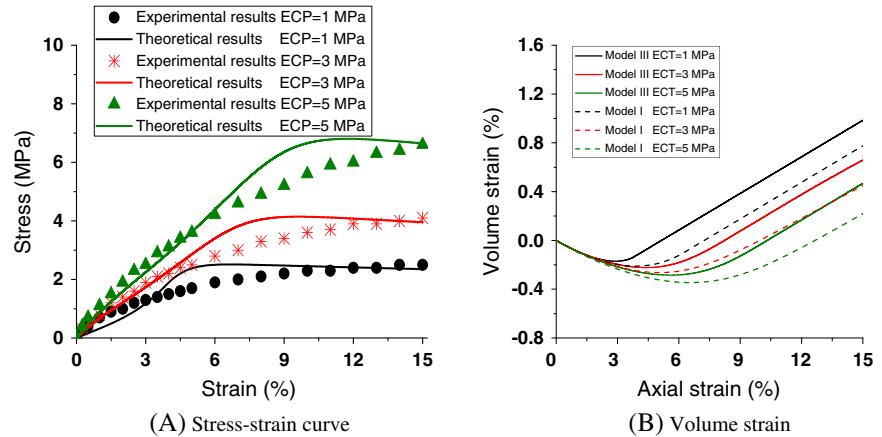


FIGURE 8 Stress-strain curves and volume strain in Model III ($S_h = 5\%$) [Colour figure can be viewed at wileyonlinelibrary.com]

Figure 8 that the stress-strain results obtained by Model III reach the peak shear strengths in lower strains than those of tests. The theoretical volume strains are larger than that by Model I.

5 | CONCLUSIONS

The mechanical properties of GHBS are investigated by the integrated experimental apparatus with triaxial compression tests and compared with the simulated results obtained by the proposed hypoplastic models. The Drucker-Prager criterion and the Mohr-Coulomb criterion are introduced into the hypoplastic model to analyze the shear strength with considering the effective porosity because of the hydrate filling in the pores of GHBS. The effects of the hydrate saturation and the effective confining pressures are studied with both experimental apparatus and the proposed hypoplastic model, and the comparison shows that the proposed hypoplastic model is capable of capturing the salient features of the mechanical properties of GHBS. Moreover, the proposed model is verified to be able to describe both the strain hardening behavior under low hydrate saturation ($\leq 40\%$) and the strain softening under high hydrate saturation ($> 40\%$) of GHBS. It is also worth noting that the empirical and experimental parameters of the model are much less than the classical elastoplastic models, and their physical meanings can be clearly related to the well-established models. The proposed model is verified to be a simple and practical reference for the evaluation of soil deformation and failure behaviors in hydrate exploration and exploitation.

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ORCID

Xuhui Zhang  <http://orcid.org/0000-0001-9614-3679>

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